



**PROGRAM** : NATIONAL DIPLOMA  
*ENGINEERING: CIVIL*

**SUBJECT** : **SOIL MECHANICS 2A**

**CODE** : **CEGA211**

**DATE** : WINTER SSA EXAMINATION 2019  
JULY 2019

**DURATION** : (X-PAPER) 08:00 – 10:00

**WEIGHT** : 40 :60

**FULL MARKS** : 70

**TOTAL MARKS** : 70

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**EXAMINER** : PROF GC FANOURAKIS

**MODERATOR** : MR F THAIMO

**NUMBER OF PAGES** : 4 PAGES AND 2 ANNEXURES

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**INSTRUCTIONS**

ANSWER ALL THE QUESTIONS.

NON-PROGRAMMABLE SCIENTIFIC CALCULATORS MAY BE USED. THE USE OF ALPHA-NUMERIC CALCULATORS IS PROHIBITED.

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**QUESTION 1**

Describe (in detail) collapsible soils and list possible options that may be implemented to found on these soils.

[8]

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**QUESTION 2**

An undisturbed soil sample was taken from one of the compacted layers of a road pavement and sent for laboratory testing.

In the laboratory, the mass of the soil was recorded as 700 g. Thereafter, it was coated with wax and the combined mass of wax and soil was found to be 720 g. The volume of the wax-coated sample was determined, by displacement, as 400 ml.

The sample was then broken open and its moisture content and particle specific gravity (G) were determined as 18 % and 2.7, respectively. The specific gravity of the wax was 0.9.

**Working from first principles**, determine the soil's:

Bulk density  
Dry density  
Void ratio  
Degree of saturation

[13]

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**QUESTION 3**

A saturated soil has a moisture content of 22 % and a dry density of  $1400 \text{ kg/m}^3$ . Determine its:

Void ratio  
Specific gravity  
Porosity  
Saturated density

[9]

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**QUESTION 4**

The following results were recorded in three liquid limit tests.

Number of taps (N)	16	23	35
Moisture Content (%)	22	19	13

- 4.1 Determine the liquid limit of the soil. (5)
- 4.2 If the plasticity limit of this soil is 10, determine the plasticity index. (2)
- 4.3 If the *in-situ* moisture content of the soil is 3 %, determine the liquidity index. Also, state how this soil is expected to behave at that *in-situ* moisture content. (4)

**[11]**

**QUESTION 5**

Using the chart provided as Annexure B, classify the following two (2) soils according to the Unified Soil Classification System (USCS).

Soil	Percentage Passing Sieve Size		Liquid Limit	Plastic Limit
	4,75 mm	0,075 mm		
<b>A</b>	20	25	29	16
<b>B</b>	99	70	45	20

**[8]**

**QUESTION 6**

When describing a soil according to the MCCSSO System, what is the relevance of describing the **Moisture, Consistency and Structure**?

**[10]**

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**QUESTION 7**

- 7.1 A laboratory compaction test requires 794 KJ/m<sup>3</sup> of energy to be imparted to a soil. The mould to be used has a diameter of 152 mm and a height of 125 mm. A hammer with a mass of 2,495 kg, which falls through a distance of 304,8 mm, is to be used.

Determine the number of layers and number of blows per layer required to impart this energy to the soil. (7)

- 7.2 The results of a number of CBR tests conducted on a soil are given below. Using these results, determine the design CBR of this material, in accordance with the Asphalt Institute Manual MS-1 Method.

CBR: 12, 19, 19, 17, 18, 21, 21, 23, 25, 18. (4)

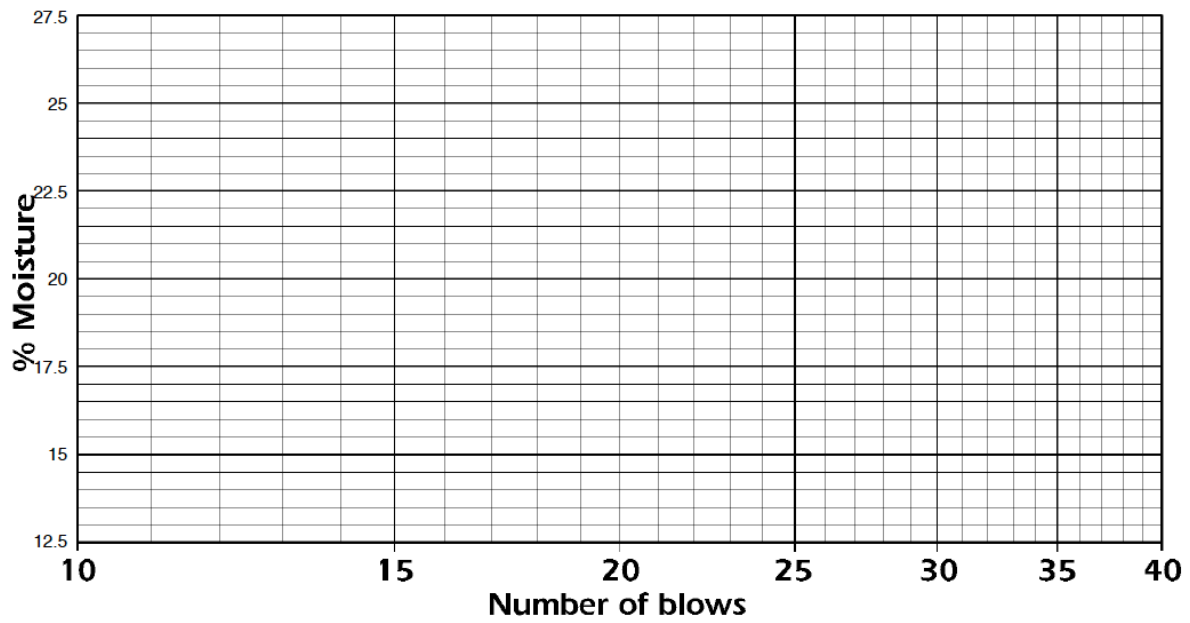
[11]

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**TOTAL : 70**

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## ANNEXURE A



## ANNEXURE B

Unified classification system

Major divisions	Group symbol	Typical names	Classification criteria for coarse-grained soils	
			Gravels (more than half of coarse fraction is larger than No. 4 sieve size)	Gravels (more than half of coarse fraction is larger than No. 200) (0.75mm)
Coarse-grained soils (more than half of material is larger than No. 200)	GW	Well-graded gravels, gravel-sand mixtures, little or no fines	Gravels with fines (little or no fines)	$C_u = D_{60}/D_{10} > 4$ $C_c = 1 < D_{30}/D_{10} \times D_{60} < 3$
	GP	Poorly graded gravels, gravel-sand mixtures, little or no fines	Gravels with fines (little or no fines)	Not meeting all gradation requirements for GW
	GM	Silty gravels, gravel-sand mixtures	Gravels with fines (little or no fines)	Atterberg limits below A line with $4 < I_p < 7$ or $I_p < 4$
	GC	Clayey gravels, gravel-sand mixtures	Gravels with fines (little or no fines)	Atterberg limits above A line with $I_p > 7$
	SW	Well-graded sands, gravelly sands, little or no fines	Gravels with fines (little or no fines)	$C_u = D_{60}/D_{10} > 6$ $C_c = 1 < D_{30}/D_{10} \times D_{60} < 3$
	SP	Poorly graded sands, gravelly sands, little or no fines	Gravels with fines (little or no fines)	Not meeting all gradation requirements for SW
	SM	Silty sands, sand-silt mixtures	Sands with fines (little or no fines)	Atterberg limits below A line with $4 < I_p < 7$ or $I_p < 4$
	SC	Clayey sands, sand-clay mixtures	Sands with fines (little or no fines)	Atterberg limits above A line with $I_p > 7$
Fine-grained soils (more than half of material is smaller than No. 200) (0.75mm)	ML	Inorganic silts and very fine sands, rock flour, silty or clayey fine sands, or clayey silts with slight plasticity	Silts and clays (liquid limit $< 50$ )	1. Determine percentages of sand and gravel from grain-size curve. 2. Depending on percentages of fines (fraction smaller than 200 sieve size), coarse-grained soils are classified as follows: Less than 5%—GW, GP, SW, SP. More than 12%—GM, GC, SM, SC 5 to 12%—Borderline cases requiring dual symbols
	CL	Inorganic clays of low to medium plasticity, gravelly clays, sandy clays, silty clays, lean clays	Silts and clays (liquid limit $< 50$ )	
	OL	Organic silts and organic silty clays of low plasticity	Silts and clays (liquid limit $> 50$ )	
	MH	Inorganic silts, micaceous or diatomaceous fine sandy or silty silts, elastic silts	Silts and clays (liquid limit $> 50$ )	
	CH	Inorganic clays of high plasticity, fat clays	Silts and clays (liquid limit $> 50$ )	
	OH	Organic clays of medium to high plasticity, organic silts	Silts and clays (liquid limit $> 50$ )	

